# PROPOSED ADDITION

## GENERAL NOTES

1. 1/150 VENTILATION MIN. AREA FOR ATTIC AND UNDER FLOOR (WHICHEVER APPLIES.)
2. FAN, IF NO WINDOW IN BATH, AND GFI REQUIRED IN ALL WET ROOMS.
3. 8% MIN. LIGHT AND 4% MIN. VENTILATION AREA IN ALL HABITABLE ROOMS, EXCEPTIONS PER IRC
4. DOUBLE FLR. JOISTS UNDER ALL PARALLEL PARTITION WALLS AND SOLID CONTINUOUS 2x SOLID BLOCKING UNDER ALL PERPENDICULAR PARTITION WALLS. ALL LOAD BEARING WALLS TO HAVE DESIGNED BEAM OR WALL UNDER.
5. FIRESTOPS IN ALL WALLS, ATTIC FLOOR CHASES, SOFFITS PER IRC
6. PRESSURE TREATED OR DECAY RESISTANT WOOD REQUIRED @ ALL CONTACT WITH CONCRETE AND EXPOSURE TO WEATHERING CONDITIONS.
7. 1/2" MIN. SHEATHING, (4' WIDE MIN. PLATE TO PLATE) OR 1 x 4 LET-IN OR APPROVED METAL STRAPS WALL BRACING REQUIRED FOR STRENGTHENING WALLS FOR MINIMUM SHEAR, THIS IS TO BE ACCOMPLISHED AT 25'-0" O.C. AND ALL CORNERS, AT ALL LEVELS WITH WOOD FRAMING. LET-INS AS CLOSE TO 45 DEGREES AS POSSIBLE.
8. TEMPERED GLASS REQUIRED WHEN SILL IS LESS THAN 18" A.F.F., 24" FROM EXT. DOOR OPENING, AND WITHIN 60" VERT. AND ABOVE TUB OR SHOWER ENCLOSURE.
9. 6'-8" MIN. HEAD CLEARANCE REQUIRED ABOVE STAIR AT ANY POINT. MIN. OF 34" HGT. HANDRAIL REQ. AT STAIR WHEN 30" OR MORE ABOVE ADJACENT LEVEL, AND 30" -38" RAIL WHEN WALLS BORDER STAIR.
10. 36" MIN. HGT. RAILING @ ALL BALCONY, PORCH, DECK OR WHERE HGT. DIFFERENCE IS 30" OR HIGHER.
11. 3'-0" MIN. ACCESS WIDTH THROUGH-OUT STRUCTURE INTERIOR, I.E. STAIR, HALL, ETC.
12. 22" x 30" MIN. ATTIC ACCESS REQUIRED.
13. 20" x 24" MIN. OPENING SIZE RQD. W/ 44" MAX. SILL HGT. AT ONE WINDOW IN EACH BEDROOM FOR EMERGENCY EGRESS. A DOOR CAN SUBSTITUTE FOR THIS EGRESS
14. 7 3/4" MAX RISER HGT. AND 10" MIN TREAD WIDTH AT ALL STAIRS.
15. 1/2" GYP. BOARD REQD. UNDER ALL STAIRS THAT USE THE AREA AS A HABITABLE ROOM.
16. 1/2" GYP. BOARD REQD. ON GARAGE SIDE OF STUDS AND JOISTS THAT ABUT A HABITABLE AREA.
17. DUAL GLAZING REQD. IF GLAZING AREA EXCEEDS 10% OF FLOOR AREA AND R-13 INSULATION REQD. IF GLAZING AREA EXCEEDS 14% OF FLOOR AREA.
18. A LIGHT GAUGE MECHANICAL CONNECTION IS REQD. AT THE BOTTOM OF ALL POST OR BUILT-UP POST, WHEN SUPPORTING A POST, BEAM, FLOOR OR ROOF STRUCTURE ABOVE, THAT CAN RESTRAIN POST FROM ANY MOVEMENT.
19. ALL CHIMNEYS TO BE 2'-0" HIGHER THAN ROOF 10'-0" AWAY HORIZONTALLY

## CODES & STANDARDS

2018 - INTERNATIONAL BUILDING CODE WITH GEORGIA AMENDMENTS (2020)
2018 - INTERNATIONAL MECHANICAL CODE WITH GEORGIA AMENDMENTS (2020)
2018 - INTERNATIONAL PLUMBING CODE WITH GEORGIA AMENDMENTS (2020)
2018 - INTERNATIONAL RESIDENTIAL CODE WITH GEORGIA AMENDMENTS (2020)
2018 - INTERNATIONAL FUEL GAS CODE WITH GEORGIA AMENDMENTS (2020)
2018 - INTERNATIONAL FIRE CODE (NO GEORGIA AMENDMENTS.)
2015 - INTERNATIONAL ENERGY CONSERVATION CODE WITH GEORGIA SUPPLEMENTS AND AMENDMENTS (2020)
2020 - NATIONAL ELECTRICAL CODE (WITH GEORGIA AMENDMENTS.)
2018 - INTERNATIONAL SWIMMING POOL AND SPA CODE WITH GEORGIA AMENDMENTS (2020)

## PROJECT DATA

Construction Type: TYPE VB - 2018 IRC
Building Height - 1-STORY
Average Ridge Height - EXIST.

## SQUARE FOOTAGE

| | |
|---|---|
| EXIST. MAIN LEVEL | 1,263 htd. s.f. |
| PROP. MAIN LEVEL | 497 htd. s.f. |
| TOTAL | 1,827 htd. s.f. |

## PROJECT TEAM

**Designer**

Garcia Residential Design
1701 Heights Circle
Kennesaw GA 30152
(678) 735-2176
fgarcia.arch@gmail.com

## INDEX OF DRAWINGS

### ARCHITECTURAL



### STRUCTURAL



PROJECT

PROPOSED ADDITION
1546 WALKER STREET
SMYRNA GA

MICHAEL QUINN
& ASSOCIATES
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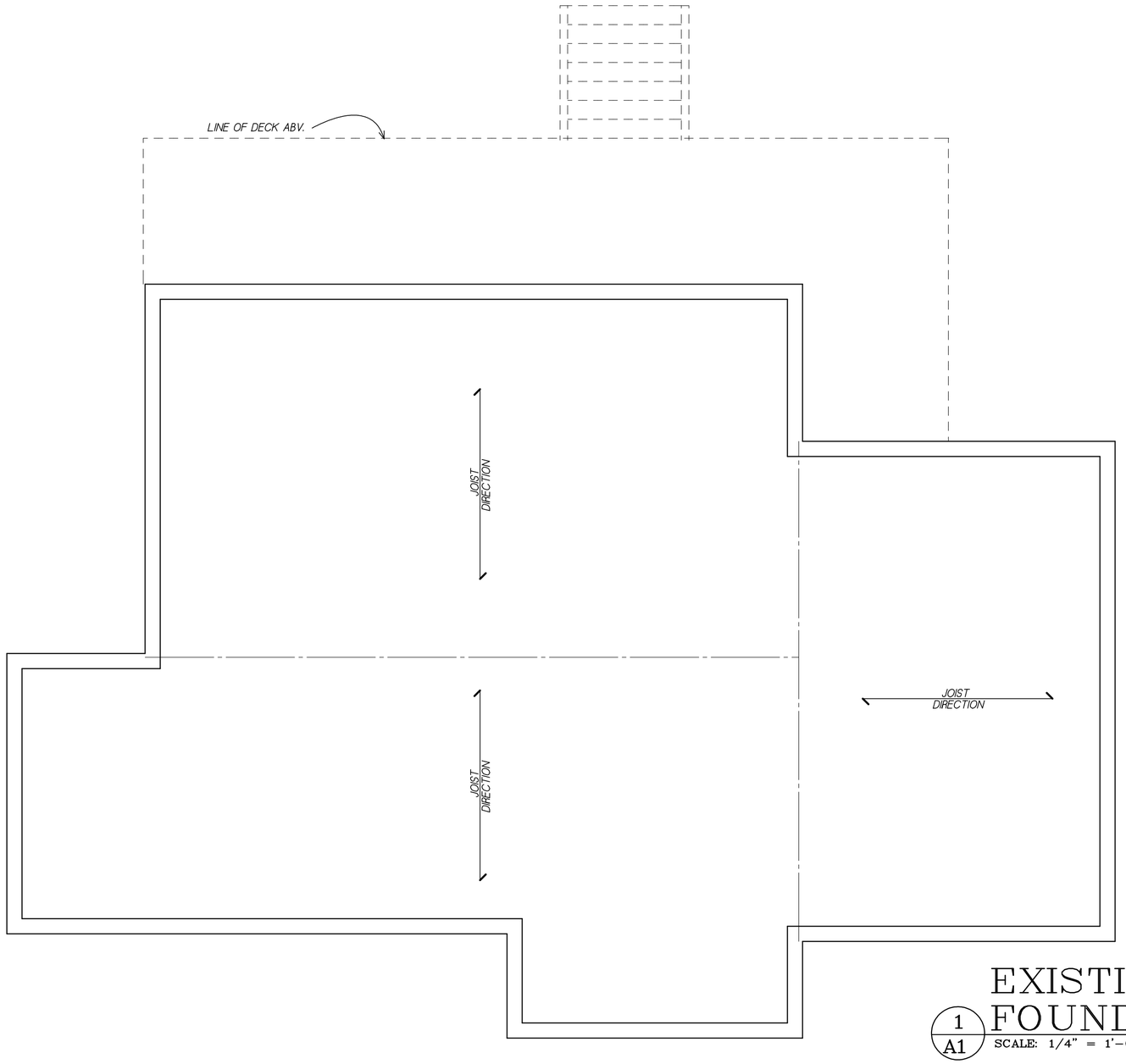
GARCIA
residential design
2115 KENNESAW DUE WEST RD KENNESAW GA 30152
TEL: (678) 735-2176



| DATE | PROJECT NUMBER |
|---|---|
| 04/05/24 | |
| DRAWN BY | REVISIONS |
| FDG | 05/13/24 |
| CHECKED BY | 05/31/24 |

RELEASED
FOR
CONSTRUCTION




# EXISTING FOUNDATION PLAN

1 / A1

SCALE: 1/4" = 1'-0"

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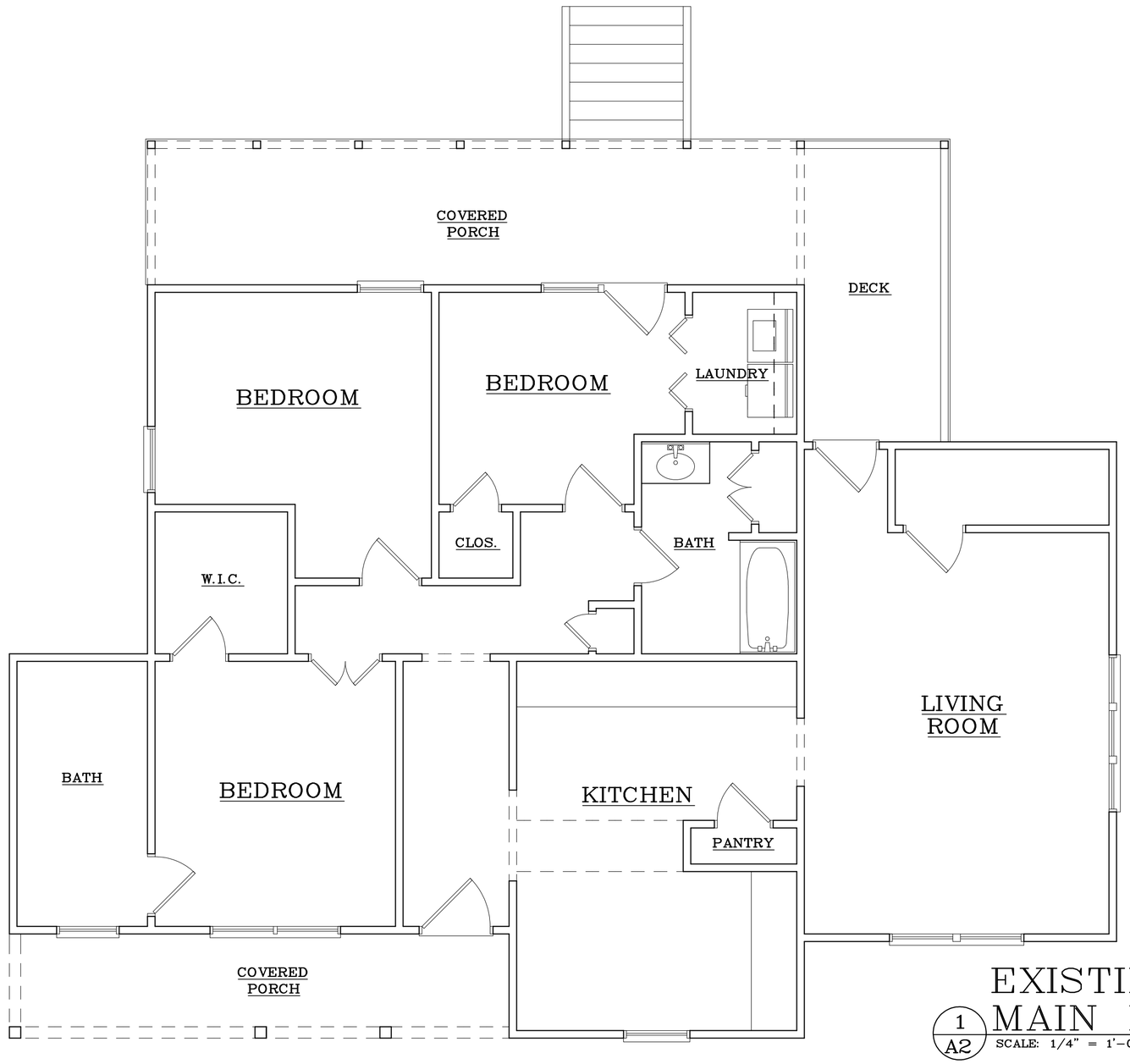


# EXISTING MAIN LEVEL PLAN

1 / A2

SCALE: 1/4" = 1'-0"

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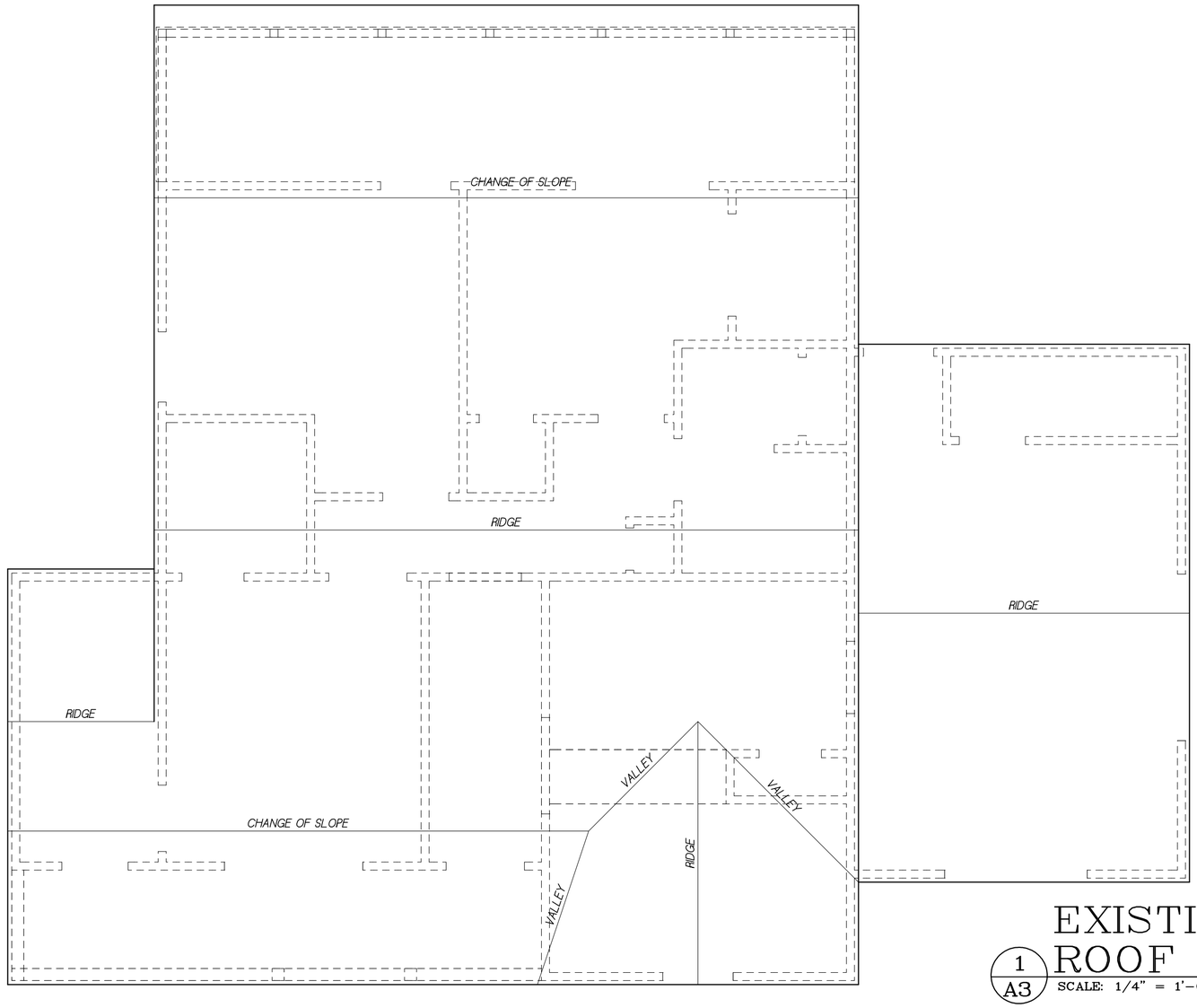


# EXISTING ROOF PLAN

1 / A3

SCALE: 1/4" = 1'-0"

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EXISTING
1 FRONT ELEVATION
A4
SCALE: 1/4" = 1'-0"

EXISTING
2 REAR ELEVATION
A4
SCALE: 1/4" = 1'-0"

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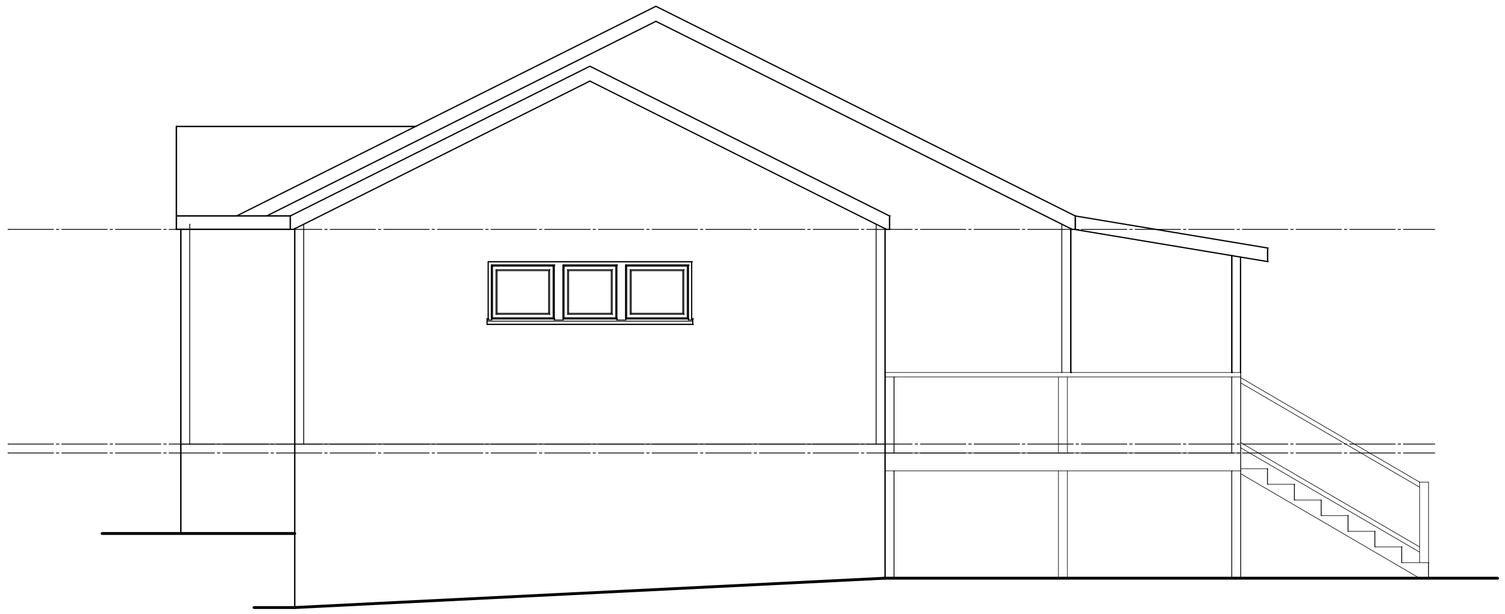
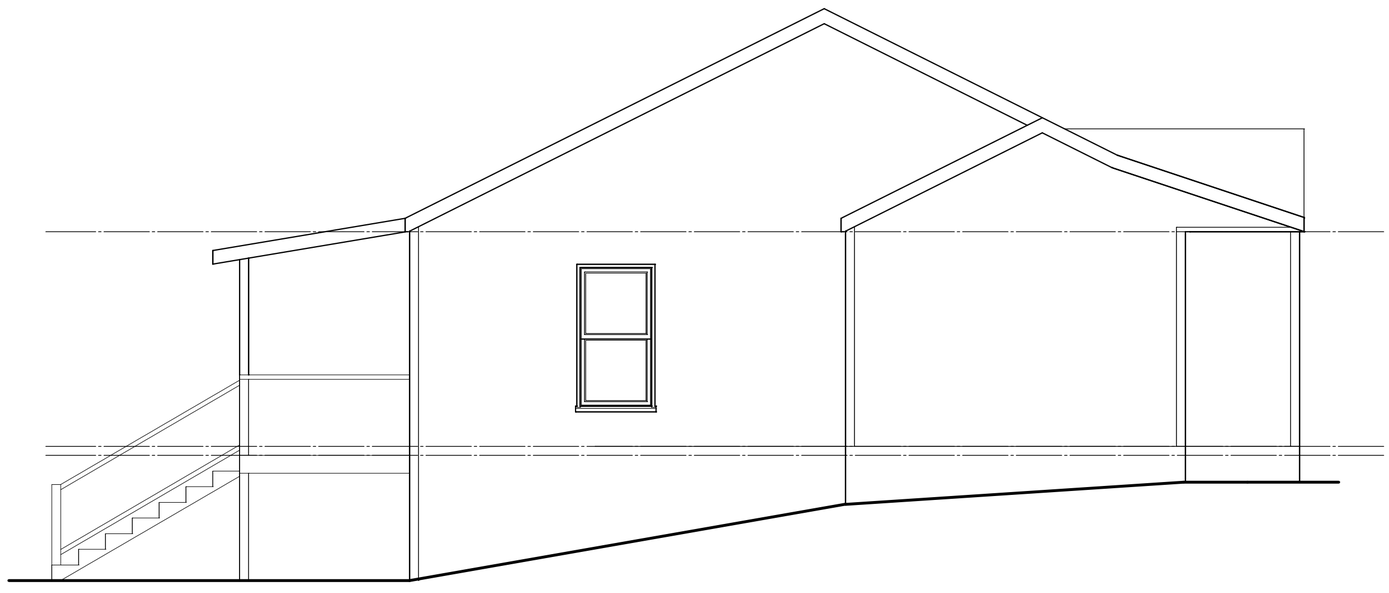


EXISTING
RIGHT ELEVATION

1 / A5

SCALE: 1/4" = 1'-0"



EXISTING
LEFT ELEVATION

2 / A5

SCALE: 1/4" = 1'-0"

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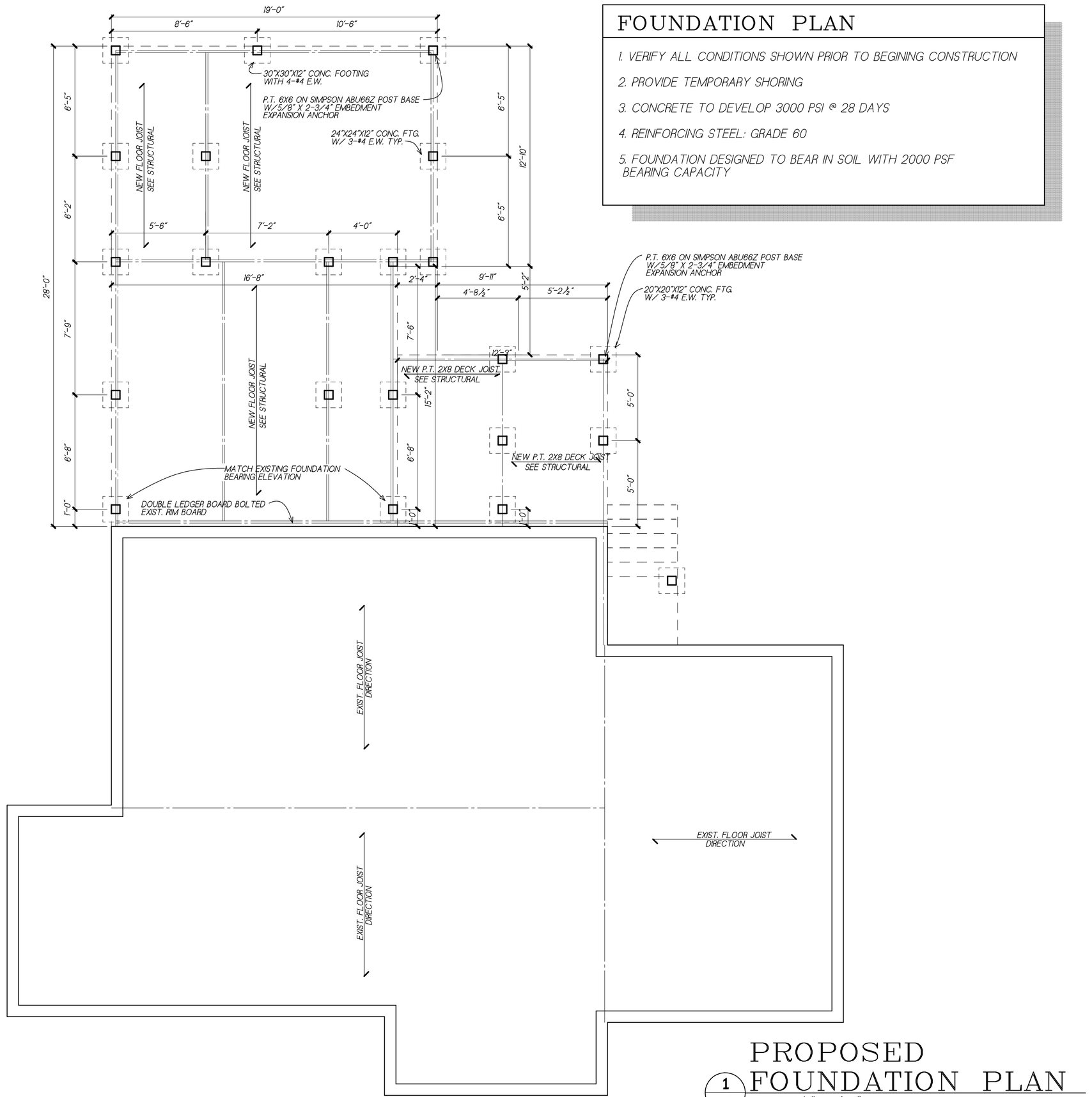
FOR
CONSTRUCTION

# FOUNDATION PLAN

1. VERIFY ALL CONDITIONS SHOWN PRIOR TO BEGINING CONSTRUCTION
2. PROVIDE TEMPORARY SHORING
3. CONCRETE TO DEVELOP 3000 PSI @ 28 DAYS
4. REINFORCING STEEL: GRADE 60
5. FOUNDATION DESIGNED TO BEAR IN SOIL WITH 2000 PSF BEARING CAPACITY




# PROPOSED FOUNDATION PLAN

1 / A6

SCALE: 1/4" = 1'-0"

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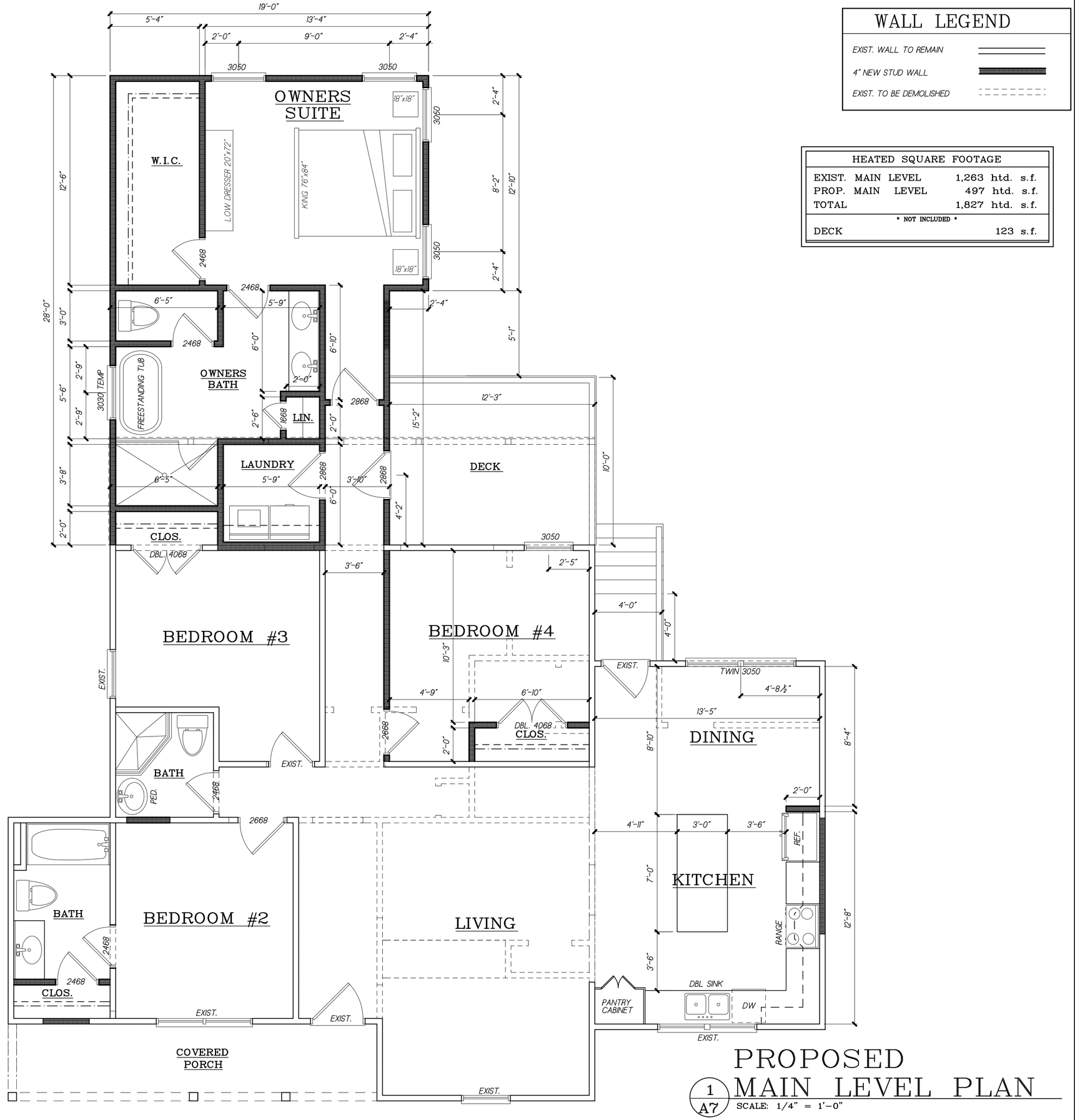
SHEET A6 of 11




# PROPOSED MAIN LEVEL PLAN

1 / A7

SCALE: 1/4" = 1'-0"

## WALL LEGEND

| | |
|---|---|
| EXIST. WALL TO REMAIN | |
| 4" NEW STUD WALL | |
| EXIST. TO BE DEMOLISHED | |

| HEATED SQUARE FOOTAGE | |
|---|---|
| EXIST. MAIN LEVEL | 1,263 htd. s.f. |
| PROP. MAIN LEVEL | 497 htd. s.f. |
| TOTAL | 1,827 htd. s.f. |
| * NOT INCLUDED * | |
| DECK | 123 s.f. |

PROJECT

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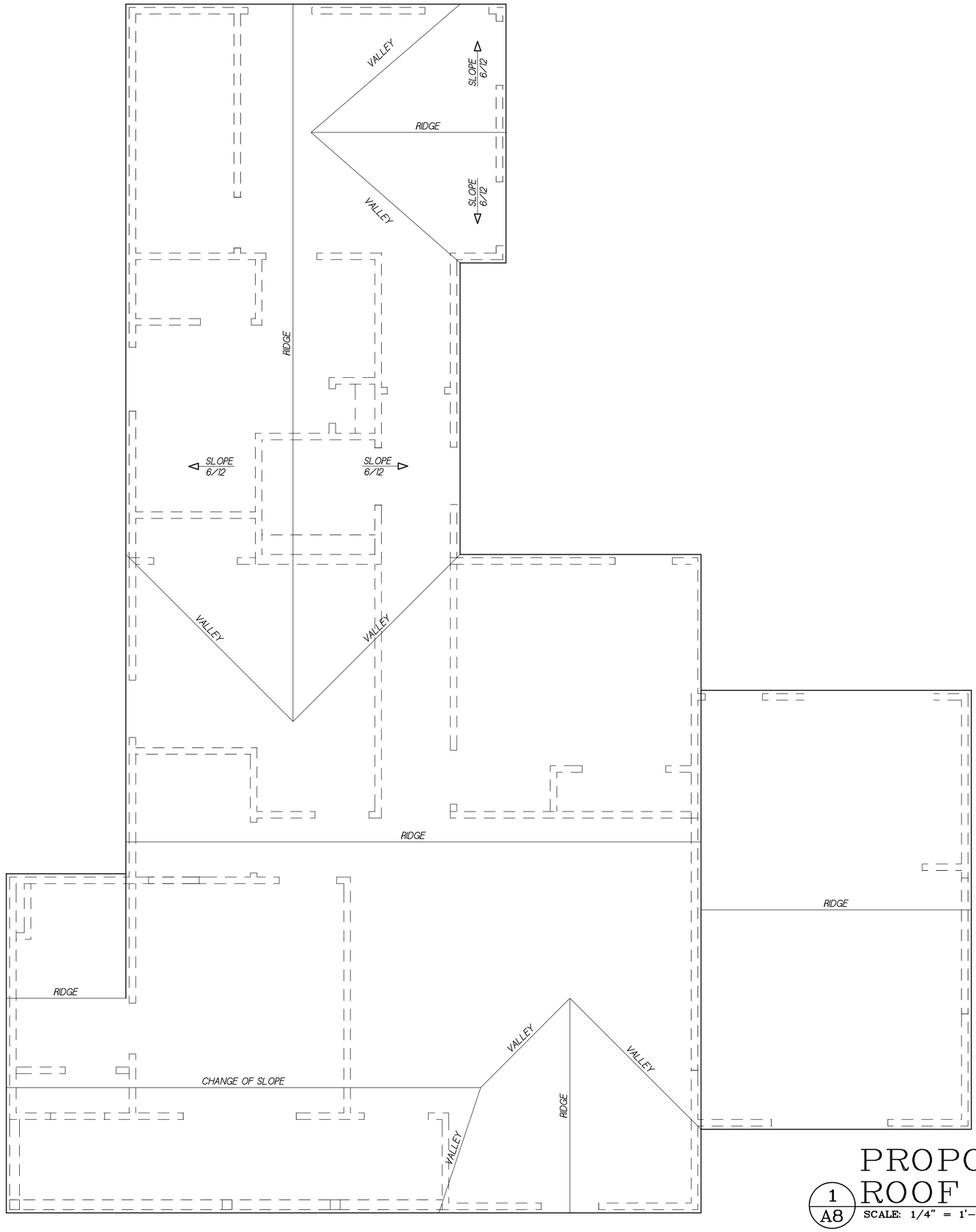
SHEET A7 of 11




# PROPOSED ROOF PLAN

1 / A8

SCALE: 1/4" = 1'-0"

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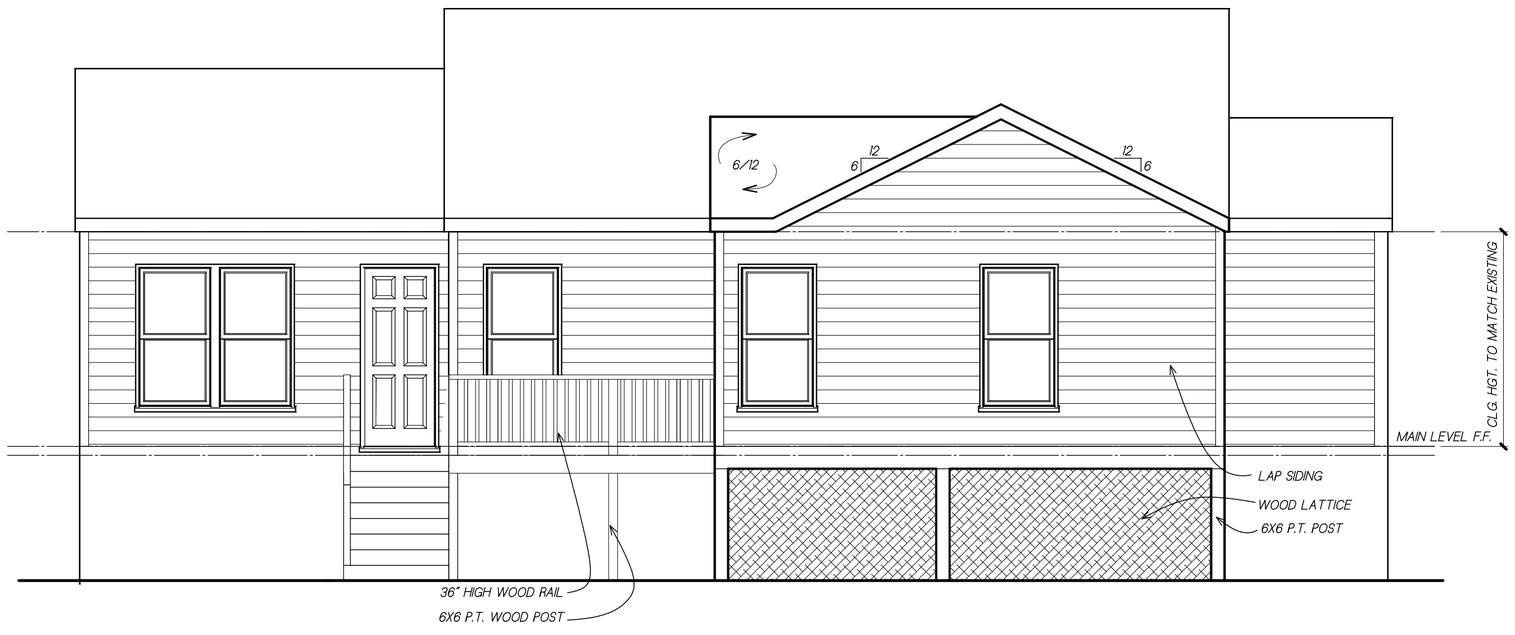
A8 SHEET 11 of

# PROPOSED FRONT ELEVATION

1 / A9

SCALE: 1/4" = 1'-0"




# PROPOSED REAR ELEVATION

2 / A9

SCALE: 1/4" = 1'-0"

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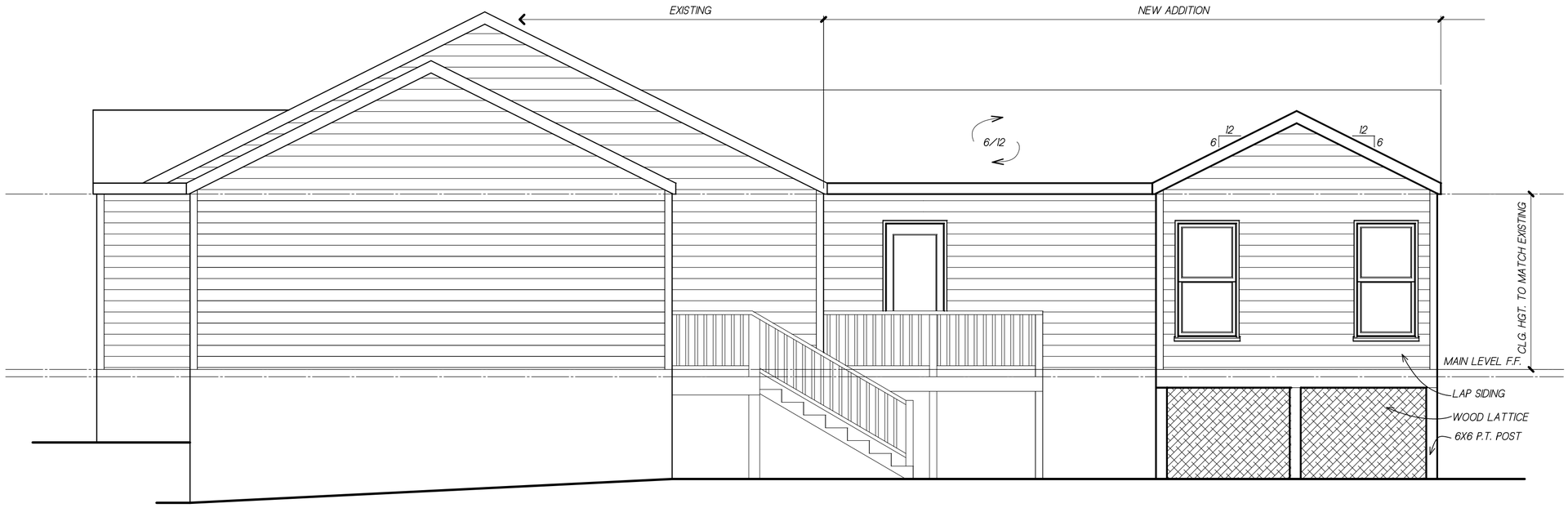
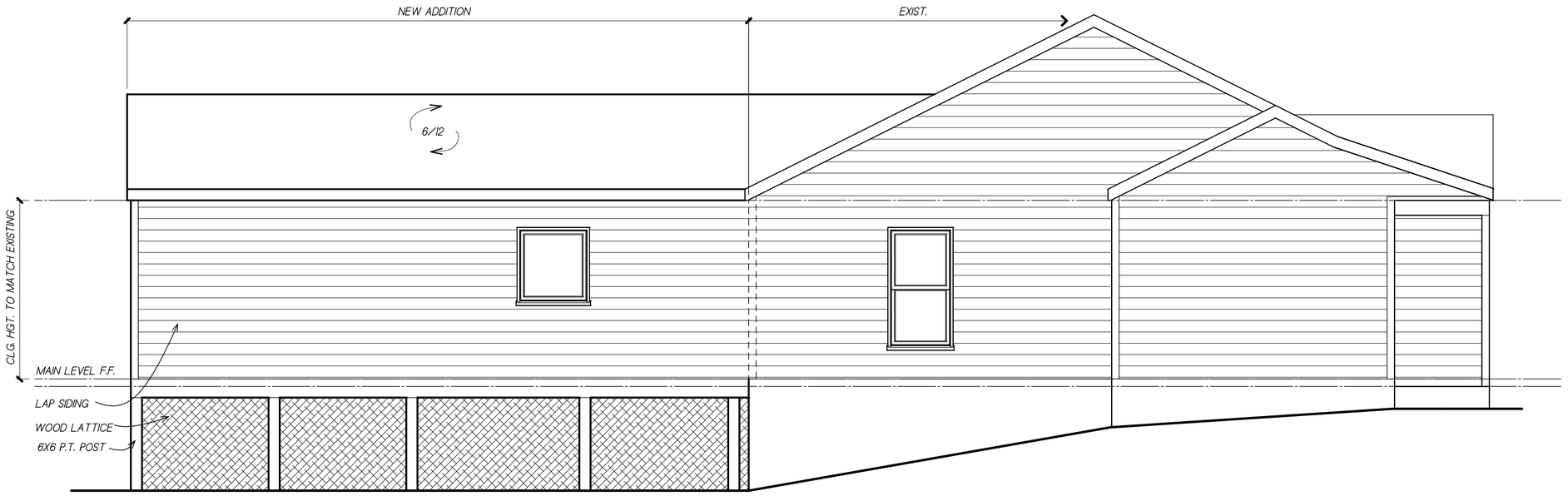
A9 SHEET 11 of




# PROPOSED RIGHT ELEVATION

1 / A10 SCALE: 1/4" = 1'-0"




# PROPOSED LEFT ELEVATION

2 / A10 SCALE: 1/4" = 1'-0"

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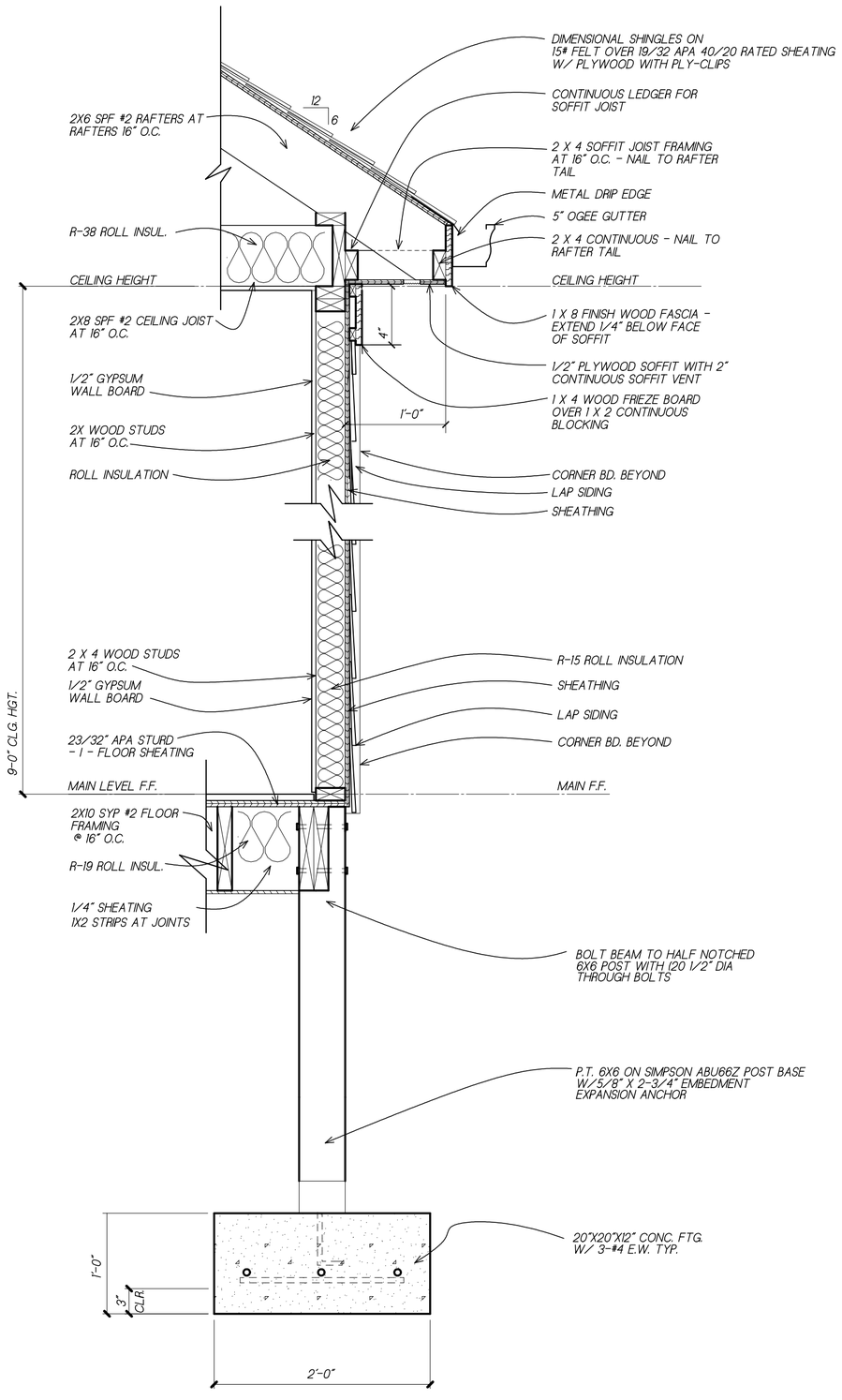
A10 SHEET 11 of




# 1 / A11 BUILDING SECTION

SCALE: 1/4" = 1'-0"

PROJECT

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CONSTRUCTION

GEORGIA REGISTERED No. 16303 PROFESSIONAL ENGINEER M. D. QUINN 4.5.2024

A11 SHEET 11 of

## GENERAL NOTES AND REQUIREMENTS

1. COORDINATE AND VERIFY ALL DIMENSIONS AND DETAILS WITH THE ARCHITECTURAL DRAWINGS. NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.
2. IF FIELD CONDITIONS VARY FROM THOSE INDICATED ON THE DRAWINGS, CONTACT THE ENGINEER BEFORE PROCEEDING
3. CONTRACTOR SHALL BE RESPONSIBLE FOR BRACING AND SHORING OF THE STRUCTURE DURING CONSTRUCTION TO ENSURE STABILITY.

## FOUNDATION NOTES

1. FOUNDATIONS ARE DESIGNED TO BEAR ON RESIDUAL SOIL OR COMPACTED FILL WITH AN ALLOWABLE BEARING CAPACITY OF 2000 PSF. BEARING CAPACITY MUST BE VERIFIED BY A GEOTECHNICAL ENGINEER PRIOR TO CONCRETE PLACEMENT. IF SOIL TEST DISCLOSE A LESSER BEARING CAPACITY, THIS FOUNDATION SYSTEM IS VOID AND MUST BE RE-DESIGNED.
2. FOOTINGS SHALL BEAR 12" MINIMUM INTO UNDISTURBED EARTH OR MECHANICALLY COMPACTED FILL.
3. FILL SOILS SHALL HAVE COMPACTION TESTS PERFORMED BY A GEOTECHNICAL ENGINEER.
4. THE GROUND IMMEDIATELY ADJACENT TO THE FOUNDATION SHALL BE SLOPED AWAY FROM THE BUILIDING AT A SLOPE NOT LESS THAN ONE UNIT VERTICAL IN 20 UNITS HORIZONTAL
5. SOILS UNDER FOOTING TO BE COMPACTED TO 98% OF THE STANDARD PROCTOR DENSITY; SOILS UNDER SLABS TO BE COMPACTED TO 95% OF THE STANDARD PROCTOR DENCITY.
6. DEPTHS OF THE FOOTINGS SHOWN ON THE DRAWINGS ARE MINIMUM. FOOTING ELEVATION MAY BE LOWERED IF FOUND NECESSARY BY THE ENGINEER.

## CONCRETE NOTES

1. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE PROJECT SPECIFICATIONS. ALL DETAILING, FABRICATION, ACCESSORIES, AND PLACEMENT OF REINFORCING SHALL CONFORM TO THE ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES. DESIGN IS IN ACCORDANCE WITH ACI318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE."
2. CONCRETE SHALL BE NORMAL WEIGHT GRAY CONCRETE AND DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, UNLESS OTHERWISE SPECIFIED.
3. REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO ASTM A615:
   GRADE 40 - #3 BARS
   GRADE 60 - #4 BARS AND LARGER
4. WELDED WIRE FABRIC REINFORCING SHALL CONFORM TO ASTM A185.
5. WHERE THE LENGTH OF A BAR IS GIVEN, AND IT IS TO BE HOOKED, THE HOOK SHALL BE IN ADDITION TO THE LENGTH GIVEN.
6. NO HORIZONTAL CONSTRUCTION JOINTS WILL BE PERMITTED IN SLABS OR WALLS.
7. ALL CONSTRUCTION JOINTS SHALL BE ROUGH SURFACE AND CLEANED BEFORE THE NEXT CONCRETE POUR.
8. WHERE OPENINGS OCCUR IN SLABS, PLACE THE REINFORCING THAT NORMALLY OCCURS IN LINE WITH THE OPENING EQUALLY TO EITHER SIDE OF THE OPENING. CUT NO STEEL IN THE FIELD.
9. MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS SHALL BE CHECKED FOR EMBEDDED ITEMS AND PENETRATIONS PRIOR TO CONCRETE PLACEMENT.
10. MINIMUM LAP SPLICE LENGTHS:
    COMPRESSION
    #4 - 15"
    #5 - 19"
    #6 - 23"
    TENSION
    #4 - 29"
    #5 - 36"
    #6 - 43"

## MASONRY NOTES

1. HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, TYPE II, GRADE N, NORMAL 16 INCH FACE SIZE. MASONRY UNITS SHALL HAVE F'M = 1500 PSI, MINIMUM.
2. MORTAR TO BE TYPE N.
3. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE 5/TMS 402).
4. MASONRY REINFORCEMENT:

   VERTICAL: PROVIDE REINFORCING BARS WHICH CONFORM TO ASTM A615 IN A SOLID GROUTED CELL OF THE SIZE AND SPACING INDICATED ON THE DRAWINGS.

   HORIZONTAL: PROVIDE 9 GAGE LADDER TYPE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY (EVERY OTHER COURSE)

5. WHERE CELLS ARE REINFORCED, FILL CELLS FULL HEIGHT OF WALL, FROM FOUNDATION TO TOP OF WALL WITH MASONRY GROUT. PLACE FILL IN LIFTS NO GREATER THAN 4'-0". MASONRY GROUT TO CONFORM TO ASTM C476. MINIMUM COMPRESSIVE STRENGTH OF GROUT TO BE EQUAL TO F'M OR 2000 PSI, WHICHEVER IS GREATER.
6. WHERE SPLICES OCCUR IN VERTICAL STEEL, LAP VERTICAL REINFORCEMENT A MINIMUM OF 48 BAR DIAMETERS, TYPICAL.
7. PROVIDE ONE #5 FULL HEIGHT VERTICAL BAR IN GROUTED CELL AT EACH SIDE OF EACH OPENING, EACH CORNER, AND WALL ENDS.
8. BRACE ALL MASONRY WALLS AS REQUIRED UNTIL ALL PERMANENT FRAMING AND ANCHORAGE ARE IN PLACE.

## STRUCTURAL STEEL NOTES

1. ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL.
2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS
   W & WT - A992 (FY = 50 KSI)
   C & MC - A36 (FY=36 KSI)
   ANGLES - A36 (FY=36 KSI)
   HSS - A500 GRADE B (FY = 46 KSI)
   PIPES - A53 TYPE S GRADE B
   PLATES AND BARS - A36 (FY = 36 KSI)
3. BOLTS SHALL BE 3/4 INCH DIAMETER A325N, WITH THREADS INCLUDED IN THE SHEAR PLANE, FOR ALL STRUCTURAL FRAMING.
4. BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED ACCORDING TO RCSC-2000 "SPECIFICATIONS FOR STANDARD JOINTS" USING ASTM A325 OR A490 BOLTS.
5. ALL BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS.
6. ALL BOLTS IN SLOTTED HOLES SHALL BE FINGER TIGHT WITH LOCK NUTS.
7. ALL WELDS SHALL BE MADE WITH E70XX ELECTRODES. ALL STRUCTURAL WELDED JOINTS SHALL CONFORM TO THE PROVISIONS OF AWS D1.1, STRUCTURAL WELDING CODE BY THE AMERICAN WELDING SOCIETY. ALL BUTT WELDS SHALL BE FULL PENETRATION. MINIMUM SIZE OF FILLET WELDS SHALL CONFORM TO AISC SPECIFICATIONS.
8. SHOP CONNECTIONS ARE TO BE WELDED CONNECTIONS, AND UNLESS OTHERWISE SHOWN ON THE DRAWINGS, FIELD CONNECTIONS ARE TO BE BOLTED.

## STRUCTURAL WOOD FRAMING NOTES

1. ALL WORKMANSHIP SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION, AND TO THE STANDARD BUILDING CODE.
2. LUMBER SIZES SPECIFIED ON THE PLANS ARE MINIMUM NOMINAL DIMENSIONS.
3. ALL LUMBER SHALL BE IDENTIFIED BY AN AFFIXED GRADE MARK OF A LUMBER GRADING AGENCY OR INSPECTING AGENCY.
4. UNLESS NOTED OTHERWISE, LUMBER FOR BEAMS, HEADERS, AND JOISTS SHALL BE SOUTHERN YELLOW PINE #2. LUMBER FOR RAFTERS SHALL BE SPRUCE-PINE-FIR #2, AND LUMBER FOR STUDS SHALL BE SPRUCE-PINE-FIR STUD GRADE.
5. FINGER JOINTED SPRUCE-PINE-FIR #2 LUMBER MEETING PRODUCT STANDARD SPS1 AND C/QC101.97 MAY BE USED FOR RAFTERS. FINGER JOINTED STUD GRADE SPRUCE-PINE-FIR LUMBER MEETING PRODUCT STANDARD SPS3 AND C/QC101.97 MAY BE USED FOR STUDS.
6. ALL LUMBER AND WOOD STRUCTURAL PANEL MEMBERS, INCLUDING PRESERVATIVE-TREATED, 2 INCH THICK AND LESS, SHALL CONTAIN NOT MORE THAN 19% MOISTURE AT THE TIME OF INSTALLATION.
7. PROVIDE PRESSURE TREATED LUMBER AT ALL LOCATIONS SPECIFIED ON THE DRAWINGS. GENERAL CONTRACTOR TO VERIFY COMPATIBILITY OF ALL METAL FASTENERS, CONNECTORS, AND HARDWARE WITH THE TYPE OF CHEMICALS USED ON ALL PRESSURE-TREATED LUMBER.
8. PROVIDE SIMPSON STRONG-TIE, OR APPROVED EQUAL, CONNECTORS AT ALL LOCATIONS SHOWN ON THE PLANS.
9. LAMINATED VENEER LUMBER (LVL) SHALL HAVE THE FOLLOWING MINIMUM DESIGN PROPERTIES, Fb = 2650 psi, Fv = 285 psi, E = 1.9E6 psi
10. WOOD I-JOISTS INDICATED "APA PRI" ARE TO CONFORM WITH THE AMERICAN PLYWOOD ASSOCIATION PERFORMANCE RATED SPECIFICATION. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
11. GLULAM BEAMS ARE TO BE 24F-V3 LAMINATION COMBINATION WITH CAMBER RADIUS OF 1600 FEET, UNLESS NOTED OTHERWISE ON THE PLAN. MEMBERS SHALL BE MARKED IN ACCORDANCE WITH ANSI STANDARD A190.1 NO HOLES OR NOTCHES ARE TO BE CUT IN GLULAMS WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.

## PRE-ENGINEERED WOOD TRUSSES

1. TRUSS DIAGRAMS, IF PROVIDED, ARE FOR CONCEPTUAL CHORD CONFIGURATIONS ONLY. MEMBER CONFIGURATION AND SIZES ARE TO BE DETERMINED BY MANUFACTURER.
2. TRUSSES SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA AND THE DRAWINGS AND CALCULATIONS SHALL BEAR HIS SEAL.
3. SUBMIT SHOP DRAWINGS FOR ALL TRUSSES TO THE ARCHITECT FOR REVEIW PRIOR TO FABRICATION. DRAWINGS SHALL SHOW TRUSS PROFILE, WEB AND CHORD MEMBER SIZES, MEMBER FORCES, AND REACTION FORCES.
4. WEB AND CHORD MEMBERS SHALL BE A MINIMUM SIZE OF 2 X 4.
5. PROVIDE CONNECTION AND INSTALLATION DETAILS FOR ALL TRUSSES, SHOWING TEMPORARY AND PERMANENT BRACING AND BRIDGING LOCATIONS IN ACCORDANCE WITH WTCA/TPI'S BCSI 1-03: GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES.
6. WHERE TRUSSES BEAR ON ANOTHER TRUSS, THE TRUSS MANUFACTURER SHALL PROVIDE ALL REQUIRED CONNECTION HARDWARE.
7. ALL TRUSS PLANS SHALL BE AVAILABLE ON THE JOB SITE DURING THE TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED AND APPROVED FOR CONSTRUCTON BY THE PROJECT ENGINEER-OF-RECORD.

## DESIGN LOADS

DESIGNED IN ACCORDANCE WITH THE 2012 INTERNATIONAL RESIDENTIAL CODE WITH GEORGIA AMENDMENTS

ROOF LIVE LOAD = 20 PSF REDUCED AS ALLOWED FOR PITCH
ROOF DEAD LOAD = 10 PSF

CEILING LIVE LOAD = 20 PSF
CEILING DEAD LOAD= 10 PSF

FLOOR LIVE LOAD = 30 PSF BEDROOMS
= 40 PSF ALL OTHER ROOMS

FLOOR DEAD LOAD = 10 PSF

BASEMENT WALLS DESIGNED FOR 45 PCF/FT EQUIVALENT FLUID PRESSURE

BASIC WIND SPEED - 90 MPH
WIND EXPOSURE - B
INTERNAL PRESSURE COEFFICIENTS - +0.18

| COMPONENT AND CLADDING LOADS | INTERIOR ZONE | END ZONE |
|---|---|---|
| 10 SF | 14.6/-15.8 | 14.6/-19.5 |
| 20 SF | 13.9/-15.1 | 13.9/-18.2 |
| 50 SF | 13.0/-14.3 | 13.0/-16.5 |
| 100 SF | 12.4/-13.6 | 12.4/-15.1 |

SEISMIC DESIGN CATEGORY B

## LEGEND

| | |
|---|---|
| | WALL BELOW LEVEL BEING FRAMED |
| | WALL ABOVE LEVEL BEING FRAMED |
| • | STACKING WALLS |
| | POINT LOAD FROM ABOVE |
| | PURLIN |
| | PURLIN BRACE |
| | JOIST OR RAFTER FRAMING DIRECTION |
| | CEILING JOIST FRAMING DIRECTION |
| | RAFTER PARTIALLY BELOW OVERBUILD |
| | BEAM / HEADER LINE |
| F.J. | FLOOR JOIST |
| C.J. | CEILING JOIST |
| DBL | DOUBLE |
| TPL | TRIPLE |
| U.N.O. | UNLESS NOTED OTHERWISE |
| W.W.F. | WELDED WIRE FABRIC |
| P.T. | PRESSURE TREATED |
| O.C. | ON CENTER |
| A.P.A. | AMERICAN PLYWOOD ASSOCIATION |
| F.PL | FLITCH PLATE |
| E.W. | EACH WAY |
| C.T. | COLLAR TIES |
| P.A.F. | POWDER ACTUATED FASTENER |

## FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

| DESCRIPTION OF BUILDING ELEMENTS | NUMBER AND TYPE OF FASTENER [a,b,c] | SPACING OF FASTENERS [d] |
|---|---|---|
| Joist to sill or girder, toe nail | 3-8d (2 1/2" X 0.113") | |
| 1" x 6" subfloor or less to each joist; face nail | 2-8d (2 1/2" X 0.113") 2 staples, 1 3/4" | |
| 2" subfloor to joist or girder, blind and face nail | 3-16d (3 1/2" X 0.135") | |
| Sole plate to joist or blocking, face nail | 16d (3 1/2" X 0.135") | 16" o.c. |
| Top or sole plate to stud, end nail | 2-16d (3 1/2" X 0.135") | |
| Stud to sole plate, toe nail | 3-8d (2 - 1/2" X 0.113") or 2-16d (3 1/2" X 0.135") | |
| Double studs, face nail | 10d (3" X 0.128") | 24" o.c. |
| Double top plates, face nail | 10d (3" X 0.128") | 24" o.c. |
| Sole plate to joist or blocking at braced wall panels | 3-16d (3 1/2" X 0.135") | 16" o.c. |
| Double top plates, minimum 24-inch offset of end joints, face nail in lapped area | 8-16d (3 1/2" X 0.135") | |
| Blocking between joists or rafters to top plate, toe nail | 3-8d (2 1/2" X 0.113") | |
| Rim joist to top plate, toe nail | 8d (2 1/2" X 0.113") | 6" o.c. |
| Top plates, laps at corners and intersections, face nail | 2-10d (3" X 0.128") | |
| Built-up header, two pieces with 1/2" spacer | 16d (3 1/2" X 0.135") | 16" o.c. along each edge |
| Continuous header, two pieces | 16d (3 1/2" X 0.135") | 16" o.c. along each edge |
| Ceiling joists to plate, toe nail | 3-8d (2 1/2" X 0.113") | |
| Continuous header to stud, toe nail | 4-8d (2 1/2" X 0.113") | |
| Ceiling joist, laps over partitions, face nail | 3-10d (3" X 0.128") | |
| Ceiling joist to parallel rafters, face nail | 3-10d (3" X 0.128") | |
| Rafter to plate, toe nail | 2-16d (3 -1/2" X 0.135") | |
| 1" brace to each stud and plate, face nail | 2-8d (2 1/2" X 0.113") 2 staples, 1 3/4" | |
| 1" x 6" sheathing to each bearing, face nail | 2-8d (2 1/2" X 0.113") 2 staples, 1 3/4" | |
| 1" x 8" sheathing to each bearing, face nail | 2-8d (2 1/2" X 0.113") 3 staples, 1 3/4" | |
| Wider than 1" x 8" sheathing to each bearing, face nail | 3-8d (2 1/2" X 0.113") 4 staples, 1 3/4" | |
| Built-up corner studs | 10d (3" X 0.128") | 24" o.c. |
| Built-up girders and beams, 2-inch lumber layers | 10d (3" X 0.128") | Nail each layer as follows: 32" o.c. at top and bottom and staggered. Two nails at ends and at each splice. |
| 2" planks | 2-16d (3 -1/2" X 0.135") | At each bearing |
| Roof rafters to ridge, valley or hip rafters: toe nail / face nail | 4-16d (3 -1/2" X 0.135") 3-16d (3 -1/2" X 0.135") | |
| Rafter ties to rafters, face nail | 3-8d (2 1/2" X 0.113") | |
| Collar tie to rafter, face nail, or 1 1/4" x 20 gage ridge strap | 3-10d (3" X 0.128") | |

| Description of Building Materials | Description of Fastener [b,c,e,g] | Spacing of Fasteners: Edges (inches) [h] | Spacing of Fasteners: Intermediate supports [c,g,i] (inches) |
|---|---|---|---|
| **Wood structural panels, subfloor, roof and wall sheathing to framing, and particleboard wall sheathing to framing** | | | |
| 5/16" - 1/2" | 6d common (2" X 0.113") nail (subfloor, wall) / 8d common (2 1/2" X 0.131") nail (roof) [f] | 6 | 12 [g] |
| 19/32" - 1" | 8d common (2 1/2" X 0.131") | 6 | 12 [g] |
| 1 1/8" - 1 1/4" | 10d common (3" X 0.148") nail or 8d (2 1/2" X 0.131") deformed nail | 6 | 12 |
| **Other wall sheathing [j]** | | | |
| 1/2" structural cellulosic fiberboard sheathing | 1 1/2" galvanized roofing nail; 8d common (2 1/2" X 0.131") nail; staple 16 ga., 1 1/2" long | 3 | 6 |
| 25/32" structural cellulosic fiberboard sheathing | 1 3/4" galvanized roofing nail; 8d common (2 1/2" X 0.131") nail; staple 16 ga., 1 3/4" long | 3 | 6 |
| 1/2" gypsum sheathing [d] | 1 1/2" galvanized roofing nail; 6d common (2" X 0.113") nail; staple galvanized 1 1/2" long; 1 1/4" screws, Type W or S | 4 | 8 |
| 5/8" gypsum sheathing [d] | 1 3/4" galvanized roofing nail; 8d common (2 1/2" X 0.131") nail; staple galvanized 1 5/8" long; 1 5/8 screws, Type W or S | 4 | 8 |
| **Wood structural panels, combination subfloor underlayment to framing** | | | |
| 3/4" and less | 1d deformed (2" X 0.120") nail or 8d common (2 1/2" X 0.131") nail | 6 | 12 |
| 7/8" - 1" | 8d common (2 1/2" X 0.131") nail or 8d deformed (2 1/2" X 0.120") nail | 6 | 12 |
| 1 1/8" - 1 1/4" | 10d common (3" X 0.148") nail or 8d deformed (2 1/2" X 0.120") nail | 6 | 12 |

a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi for shank diameter of 0.192 inch (pneumatic common nail), 90 ksi for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi for shank diameters of 0.142 inch or less.
b. Staples are 16 gauge wire and have a minimum 7/16 inch on diameter crown width.
c. Nails shall be spaced not more than 6 inches on center at all supports where spans are 48 inches or greater.
d. Four-foot-by-8-foot or 4-foot-by-8-foot panels shall be applied vertically.
e. Spacing of fasteners not included in this table shall be based on Table R602.3(2).
f. For regions having basic wind speed of 110mph or greater, 8d deformed (2 1/2" X 0.120") nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable end walls, if mean roof height is more than 25 feet, up to 35 feet maximum.
g. For regions having a basic wind speed of 100mph or less, nails for attaching wood structural panel roof sheathing to gable end wall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100mph, nails for attaching panel roof sheathing to intermediate shpporrs shall be spaced 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.
h. Gypsum sheathing shall conform to ASTM C 79 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208.
i. Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and required blocking and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and required blocking. Blocking of roof or floor sheathing panel edges perpendicular to the framing members need not be provided except as required by other provisions of the code. Floor perimeter shall be supported by framing members or solid blocking.

PROJECT

PROPOSED ADDITION
1546 WALKER STREET
SMYRNA GA


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TEL: (678) 735-2176




| DATE | PROJECT NUMBER |
|---|---|
| 04/05/24 | |
| DRAWN BY: FDG | REVISIONS: 05/13/24, 05/31/24 |
| CHECKED BY | |

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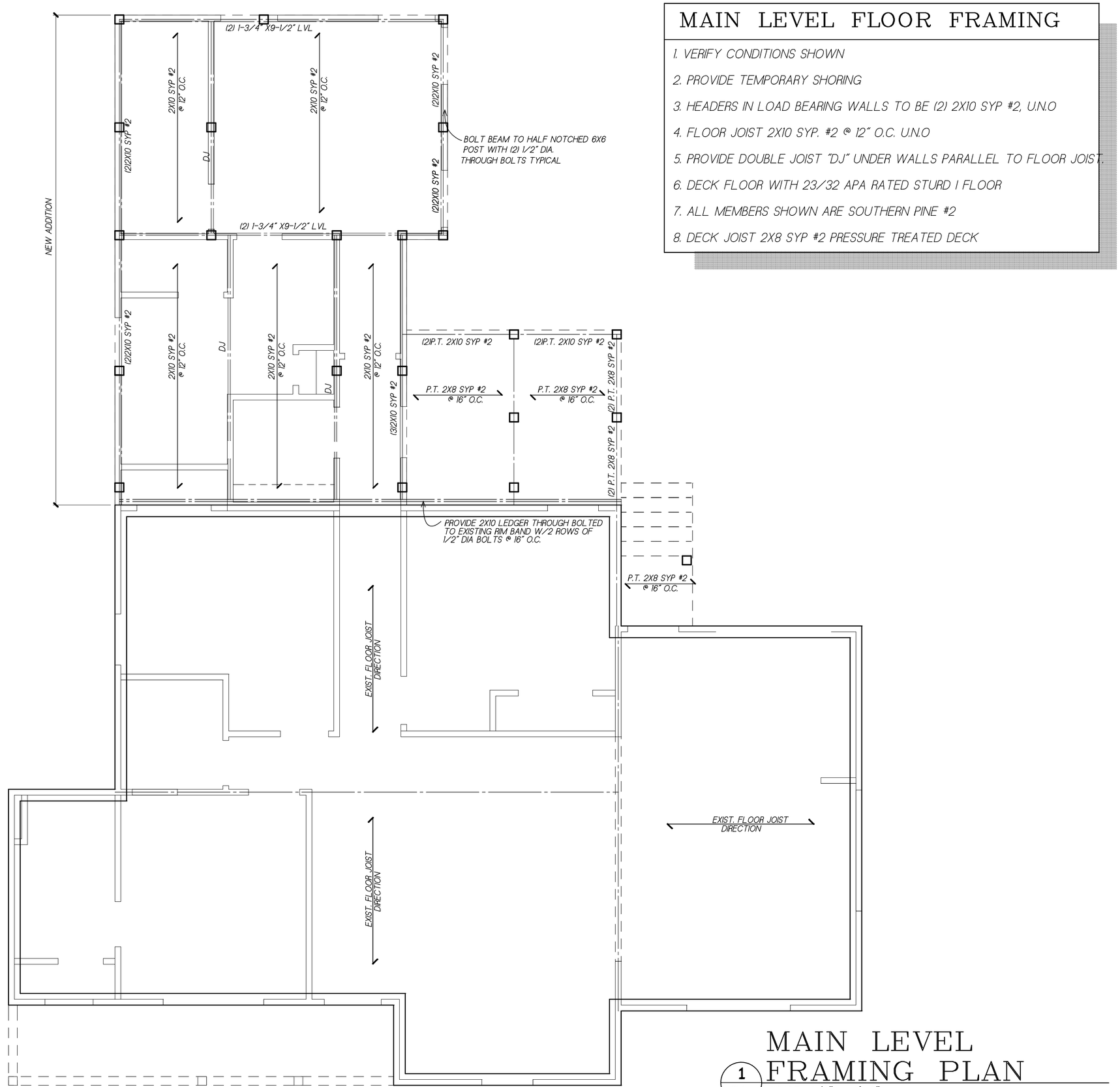
GEORGIA REGISTERED PROFESSIONAL ENGINEER M. D. QUINN No. 16103 4.5.2024

S0 SHEET 11 of




## MAIN LEVEL FLOOR FRAMING

1. VERIFY CONDITIONS SHOWN
2. PROVIDE TEMPORARY SHORING
3. HEADERS IN LOAD BEARING WALLS TO BE (2) 2X10 SYP #2, U.N.O
4. FLOOR JOIST 2X10 SYP. #2 @ 12" O.C. U.N.O
5. PROVIDE DOUBLE JOIST "DJ" UNDER WALLS PARALLEL TO FLOOR JOIST.
6. DECK FLOOR WITH 23/32 APA RATED STURD I FLOOR
7. ALL MEMBERS SHOWN ARE SOUTHERN PINE #2
8. DECK JOIST 2X8 SYP #2 PRESSURE TREATED DECK

# MAIN LEVEL FRAMING PLAN

1 / S1

SCALE: 1/4" = 1'-0"

PROJECT

PROPOSED ADDITION
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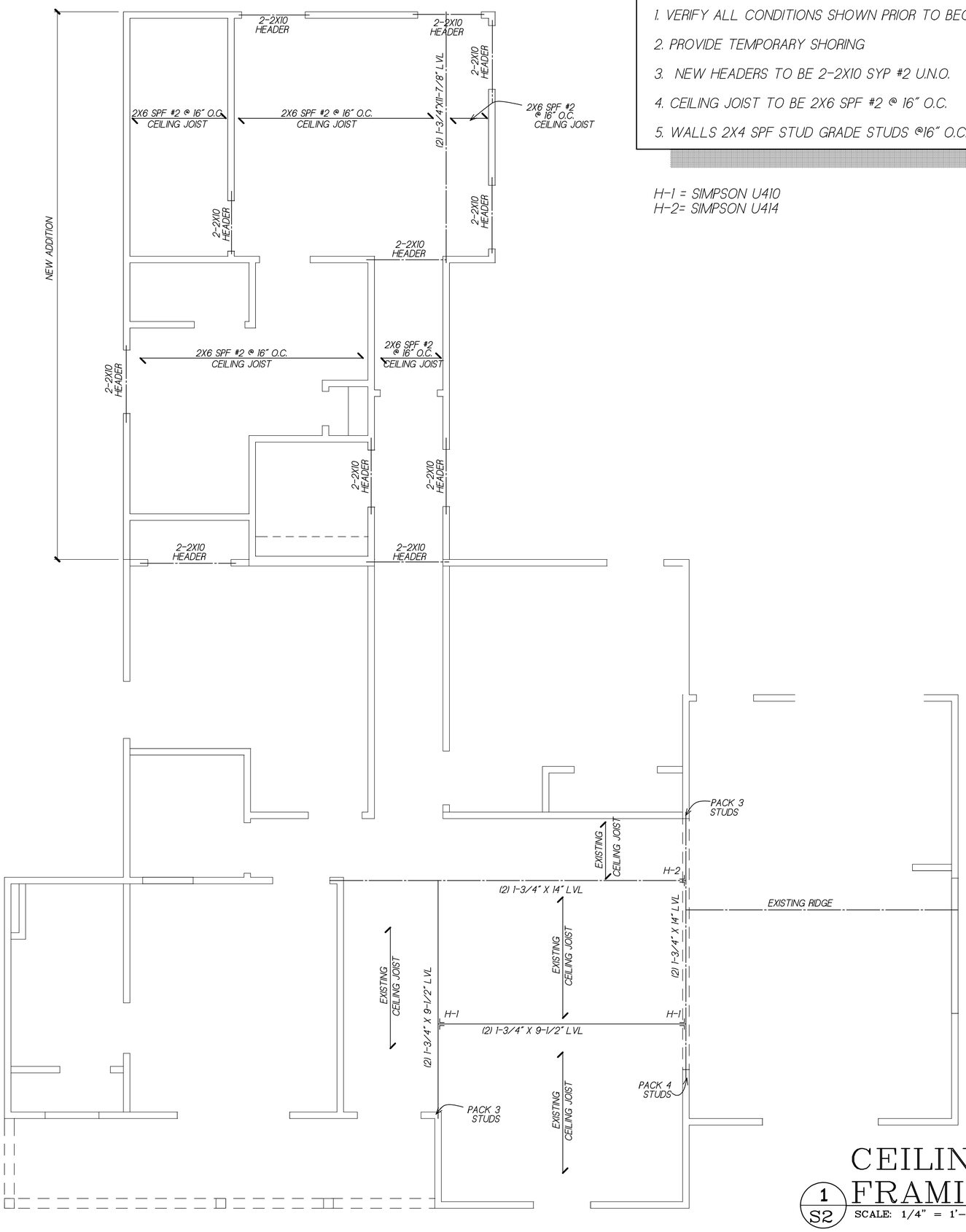
S1 SHEET 11 of

# CEILING FRAMING PLAN

1. VERIFY ALL CONDITIONS SHOWN PRIOR TO BEGINING CONSTRUCTION
2. PROVIDE TEMPORARY SHORING
3. NEW HEADERS TO BE 2-2X10 SYP #2 U.N.O.
4. CEILING JOIST TO BE 2X6 SPF #2 @ 16" O.C.
5. WALLS 2X4 SPF STUD GRADE STUDS @16" O.C.

H-1 = SIMPSON U410
H-2= SIMPSON U414




## 1 S2 CEILING FRAMING PLAN

SCALE: 1/4" = 1'-0"

PROJECT

PROPOSED ADDITION
1546 WALKER STREET
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SHEET S3 of 11

# ROOF FRAMING PLAN

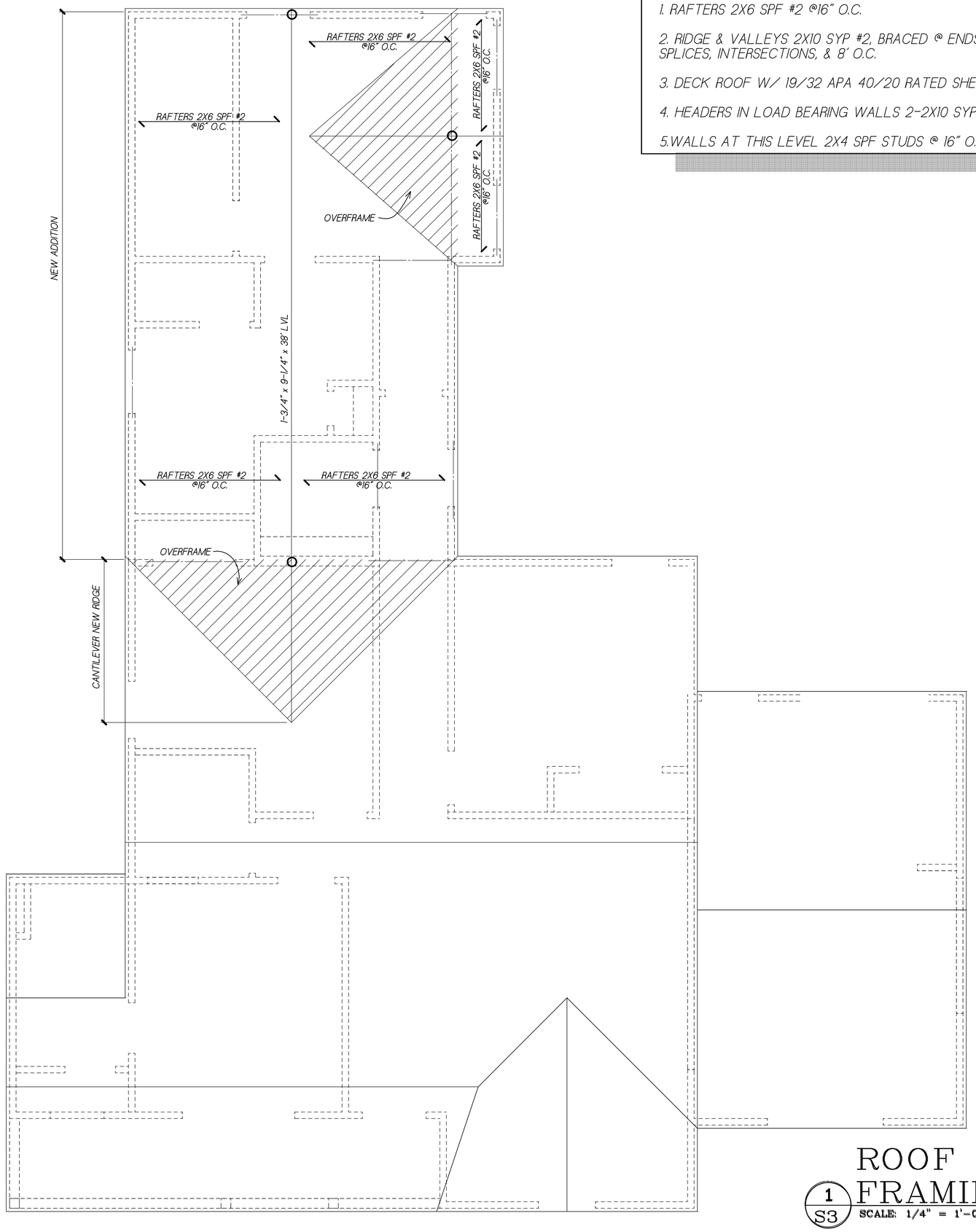
1. RAFTERS 2X6 SPF #2 @16" O.C.

2. RIDGE & VALLEYS 2X10 SYP #2, BRACED @ ENDS, SPLICES, INTERSECTIONS, & 8' O.C.

3. DECK ROOF W/ 19/32 APA 40/20 RATED SHEATHING

4. HEADERS IN LOAD BEARING WALLS 2-2X10 SYP #2 U.N.O

5.WALLS AT THIS LEVEL 2X4 SPF STUDS @ 16" O.C.




## ROOF FRAMING PLAN

1 / S3

SCALE: 1/4" = 1'-0"

PROJECT

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GEORGIA REGISTERED PROFESSIONAL ENGINEER No. 16103 M. D. QUINN 4.5.2024

SHEET S1 of 11